

Deterministic and Probabilistic Earthquake Scenarios for the Seismic Risk Analysis of URM Buildings

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Abstract. Barcelona, as well as a large number of cities in the Mediterranean basin, has a housing stock composed of a large number of unreinforced brick masonry buildings. Motivated by different factors, the enlargement of the city (Eixample in Catalan) was held from the second half of the 19th century and the beginning of the 20th, a period in which a large number of buildings of this type were built, many of which are still used as dwellings.

Although the buildings were built individually, some of them are linked to adjacent buildings by the side walls. This feature leads to the analysis of the buildings as isolated structures and also as an aggregate.

Barcelona is located in a seismic region of low to moderate hazard, with macroseismic intensity between the grades VI and VII of the European macroseismic scale EMS'98. Based on the deterministic and probabilistic response spectra for the different types of soils present in Barcelona obtained in the work of Irizarry (2004), the seismic risk of four individual buildings and an aggregate is evaluated. The buildings are modeled and analyzed using the TREMURI program and MATLAB routines under the guidance of RISK-UE project.

Introduction

The modern cities of the Mediterranean basin accumulate a large stock of infrastructures, resulting in a concentration of important socioeconomic value. The construction period, typology, materials and code levels in an important number of buildings of this stock, lead us to evaluate the seismic hazard and vulnerability of some of them.

In the case of Barcelona city, originally founded by Romans between the 15 to 10 years B.C., the enlargement of the city (example in Catalan) took place during the second half of the 19th century and the beginning of the 20th century. During this period of expansion, most of the buildings were constructed with unreinforced brick masonry and without any seismic consideration. Nowadays, many of those ancient buildings are still used as dwellings, largely surpassing their initially supposed service life.

Sometimes, the constructive habits of the time allowed the construction of the buildings not only as isolated structures but also as buildings assembled to each other as aggregates, constituting the so called blocks.

Following the guidance of the RISK-UE project [1], the aim of this work is to assess the seismic risk and vulnerability of the isolated and aggregated buildings. The structural analysis is performed with the TREMURI [2] program and some MATLAB (Matlab, v.2009b, The Mathworks), routines that allow us to obtain the different results concerning fragility and damage.

Based on the work and results obtained by Irizarry (2004), we have been able to analyze the buildings for a deterministic and a probabilistic earthquake scenario for the corresponding type of soil where these buildings are located.

The buildings

The studied buildings correspond to a block section located in Muntaner Street in the city of Barcelona, Spain. The four isolated buildings correspond to the numbers 153, 155, 157 and 159, and the aggregate corresponds to the buildings with the number 153 and 155, whom happen to be identical (Fig. 1).

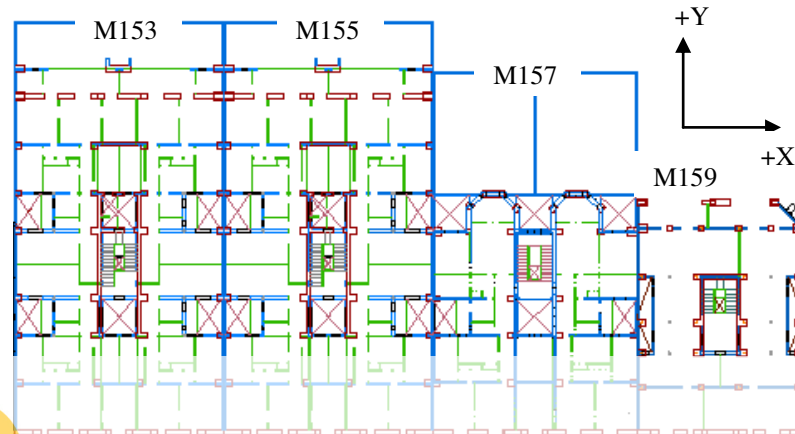


Fig. 1 Floor plan of the analyzed buildings

The buildings are quite representative of the typology of buildings existent in the Eixample district, presenting bearing walls as main resistant elements and, in some cases in the base levels, masonry or cast iron columns. The evaluated structures have 7 levels.

The façade walls present thickness from 45 or 60 cm in the base level, to 30 cm in the upper levels. In the case of the side walls the thickness of the first level is 30 cm, being reduced to 15 cm for the consequent levels. The internal walls tend to present the same thickness than the side walls in each level including the staircases core walls (Fig. 2). There are some distribution walls which structural function is neglected due to their low thickness of 10 cm or lower.

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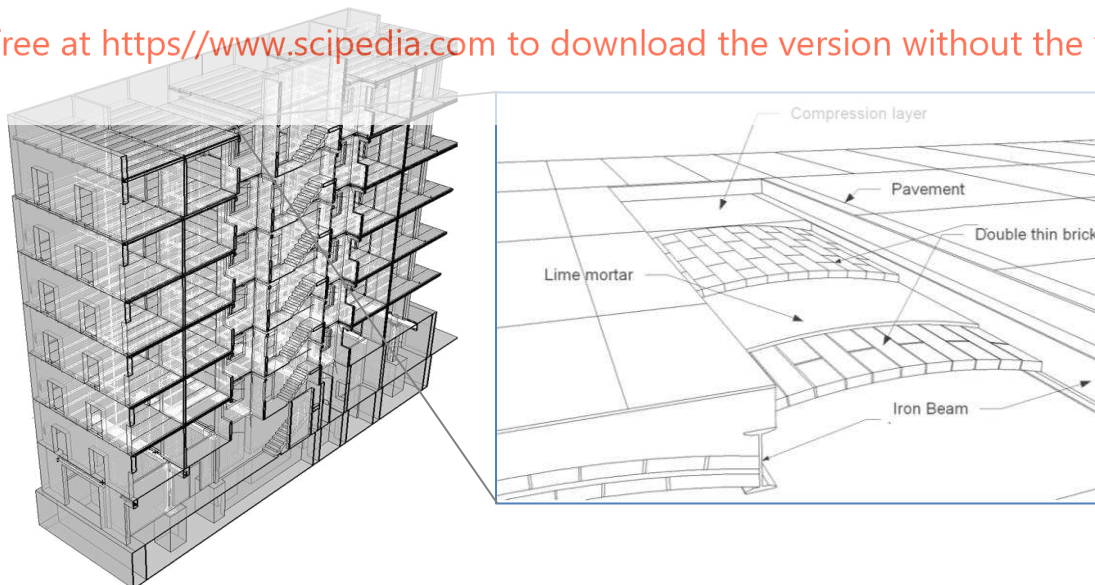


Fig. 2 Section cut of one of M153 building and its floor system

We can observe in the different type of walls the presence of big openings that work as doorways or windows, which loads are discharged via iron or wood lintels or via arches. In the case of the floors, for this particular construction period it is usual to find one-way timber floors, wood beams and brick vaults or, like in our buildings, iron beams and brick vaults (Fig. 2). The size and separation of the beams depends on the total distance that they cover, usually having separations between 60 cm and 120 cm.

The demand

Barcelona is located in a low-to-moderate seismic region. The data related to the demand used in this work includes the 5 percent-damped demand spectra proposed in the RISK-UE project, originally deduced by the University of Genoa [3]. The different soil types present in the city and the main features for the acceleration response spectra for the deterministic and probabilistic earthquake scenarios were obtained from the previous studies performed by Cid (1998) and Irizarry (2004). The analyzed buildings are located in the Eixample neighborhood, which is placed in the soil zone II.

Soil Zone	Parameters							Scenario
	pga [g]	T_B	T_C	B_C	d	T_D	B_D	
II	0.194	0.10	0.22	2.45	1.43	2.20	0.09	Deterministic
	0.141	0.10	0.23	2.50	1.28	2.21	0.14	Probabilistic

Table 1. Parameters for the deterministic and probabilistic scenarios [4]

The capacity and fragility

The procedure followed in order to assess the capacity and fragility is based on the capacity spectrum method, which provides a graphical representation of the structure's force-displacement capacity curve and compares it to the response spectra representations of the seismic demand [5, 6].

The RISK-UE guidelines [1] define an undamaged state (NO) and four damage states, d_{si} , which are: Slight (SL), Moderate (MO), Severe (SE) and Collapse (CO). For the purpose of this work, the chosen parameter in order to define the seismic action will be the spectral displacement, S_d , and the damage states are defined from the parameters of the bilinear representation of the capacity spectrum, as follows: $\overline{ds}_1 = 0.7D_y$; $\overline{ds}_2 = D_y$; $\overline{ds}_3 = D_y + 0.25(D_u - D_y)$; and $\overline{ds}_4 = D_u$, where, D_y is the displacement at the yielding point and D_u is the displacement at the ultimate point.

It is assumed that the fragility curves will follow a lognormal cumulative probability function and that the seismic damage of the buildings will follow a binomial probability distribution (or an equivalent beta distribution). It is also assumed that for each damage state, the probability of reaching or exceeding that particular damage state will be 50%.

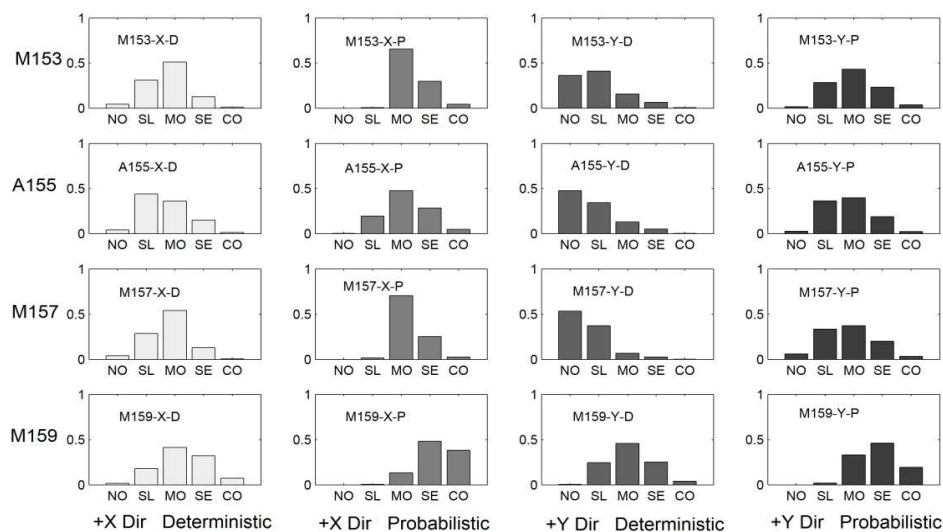


Fig. 3 Damage states distributions for each analyzed building, direction and scenario

Following the mentioned methodology and assumptions we can observe in Fig. 3 the damage states distributions for each building in its two directions, +X and +Y, and for both seismic scenarios, deterministic and probabilistic.

Discussion and Conclusions

For both directions, the probabilistic scenario presents higher damage values and worst performance than the deterministic scenario, in consistency with the corresponding *pga* values. For both scenarios, we can observe a better performance in the +Y direction rather than in the +X direction (short direction).

In the case of the aggregate buildings, and comparing the results to those obtained for the isolated building M153, we can observe a significant improvement especially in the +Y direction, suggesting a positive effect for this construction habit.

For the rectangular plan buildings (M153 and M157) we can notice that the damage state probabilities, in the +X direction, are centered between the slight and moderate states for the deterministic scenario, and between moderate and severe for the probabilistic scenario, meanwhile for +Y direction the damage is centered between the no damage and the slight states, for the deterministic scenario, and between slight and moderate for the probabilistic scenario. Buildings with a more squared floor plan (M159) show closer values of fragility.

The results indicates that the expected damage associated to these unreinforced masonry buildings of Barcelona is predominantly moderate

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References

- [1] Z. V. Milutinovic and G. S. Trendafiloski, "WP4: Vulnerability of Current Buildings," in *RISK-UE Project Handbook*, 2003, p. 111.
- [2] S. Lagomarsino, A. Galasco, A. Penna and S. Cattari, *TREMURI: Seismic Analysis Program for 3D Masonry Buildings (User Guide)*, Technical Report ed., U. o. Genoa, Ed., Genoa, Italy, 2008.
- [3] S. Lagomarsino, A. Galasco and A. Penna, "Pushover and dynamic analysis of URM buildings by means of a non-linear macro-element model," in *International Conference on Earthquake Loss Estimation and Risk Reduction*, Bucharest, 2002.
- [4] J. Irizarry, X. Goula and T. Susagna, "Evaluación de la peligrosidad sísmica de la ciudad de Barcelona en términos de aceleración espectral," in *2o Congreso Nacional de Ingeniería Sísmica*, Málaga, Spain, 2003.
- [5] ATC-40, "Seismic Evaluation and Retrofit of Concrete Buildings," Seismic Safety Commission, Redwood City, CA, 1996.
- [6] P. Fajfar and P. Gaspersic, "The N2 Method for the seismic damage analysis of RC buildings," *Earthquake Engineering & Structural Dynamics*, vol. 25, no. 1, pp. 31-46, 1998.
- [7] FEMA/NIBS, *HAZUS Technical Manual SR2*, Vols. 1, 2, 3, Federal-Emergency-Management-Agency and National-Institute-of-Building-Sciences, Eds., Washington, 2002.
- [8] J. Cid, "Zonación Sísmica de la ciudad de Barcelona basada en métodos de simulación numérica de efectos locales," Universitat Politècnica de Catalunya, Barcelona, Spain, 1998.
- [9] J. Irizarry, "An Advanced Approach to Seismic Risk Assessment. Application to the Cultural Heritage and the Urban System Barcelona," Universidad Politècnica de Cataluña, Barcelona, Spain, 2004.
- [10] A. J. Kappos, "Seismic damage indices for RC buildings: evaluation of concepts and procedures," *Progress in Structural Engineering and Materials*, vol. 1, no. 1, pp. 78-87, 1997.

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